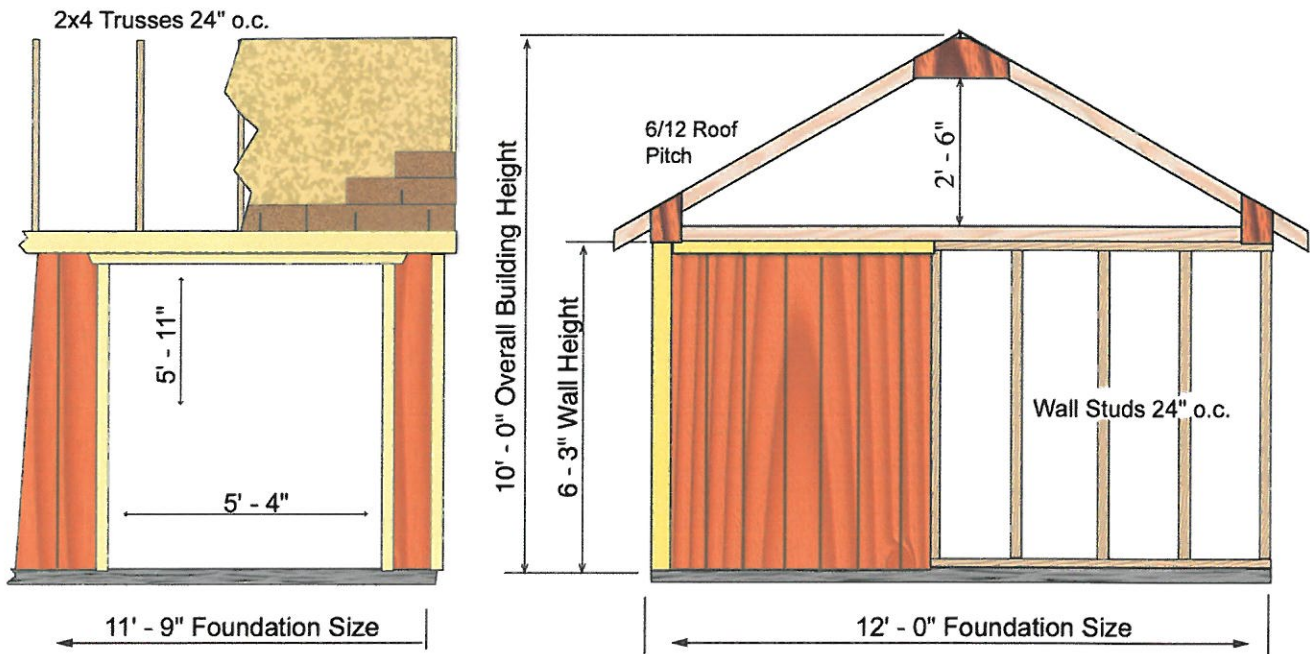




Before you order our kit or begin construction, obtain a building permit. The information below and the attached truss drawing should provide the information you will need.

If additional documents are required contact Richard@barnkits.com.

## MANSFIELD ELEVATION



### GENERAL SPECIFICATIONS

**Foundation:** By owner

**Wall Framing:** Constructed from 2x4 pre-cut wall studs spaced 24" on center between top and bottom plates.

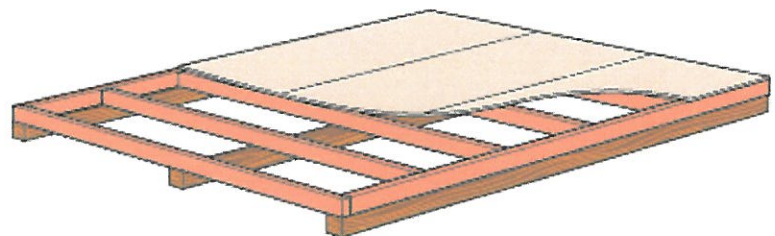
**Siding:** Louisiana-Pacific 'Smart Panel' primed 8" o.c. groove with 50 year warranty, 5 year labor replacement

**Roof System:** 2x4 trusses spaced 24" on center, (40 psf ground snow load, 130 mph wind load). 7/16" OSB roof sheathing. *Shingles by owner.*

**Exterior Trim:** White pine trim for corners, door, gable trim and front and back fascia.

**Hardware:** Nails for all framing, metal hurricane hangers for trusses.

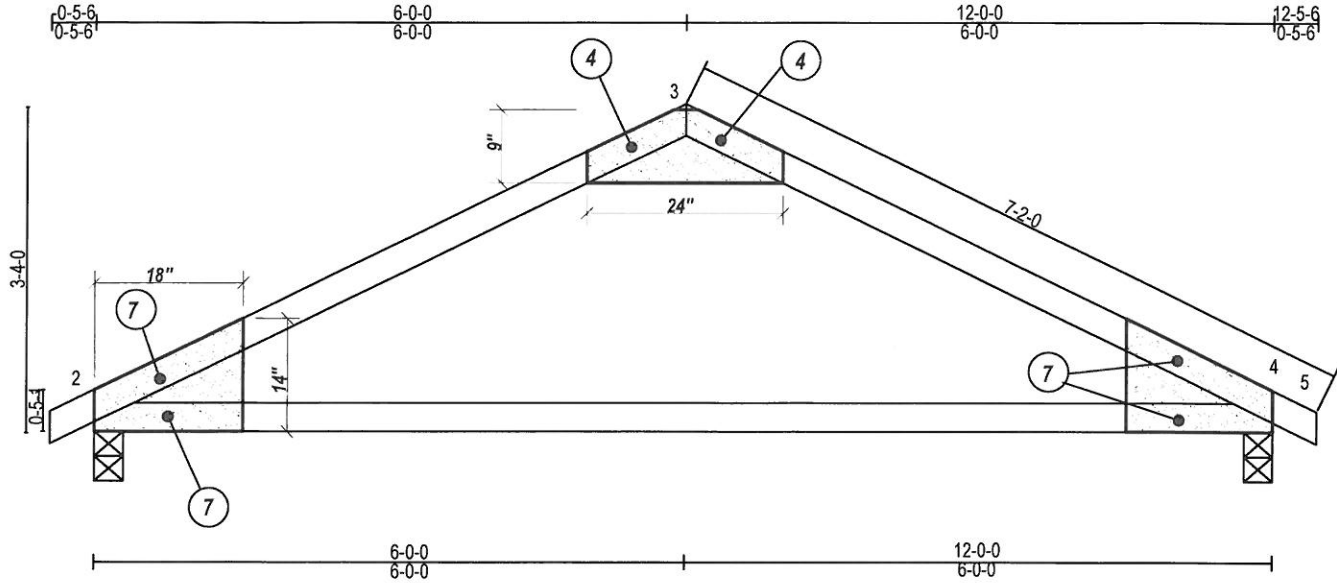
**Optional Floor:** 2x4 treated floor joist spaced 16" on center covered with 5/8" structurwood, installed over 4x4 treated runners. Note: 12' wide floors have four (4) treated runners. Nails are included. Material is not pre-cut.



Job	Truss	Truss Type	Qty	Ply	EZup Sheds - Sentry Building
10-0233_HALLMARK	T01	COMMON	1	1	

Koehlinger Engineering, Bolivar, OH 44612, (KDH)

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7/16" oriented strand board (OSB) glued and nailed to both faces of the truss using sizes specified.



Quantity of 6d nails (.113 x 2") to be staggered into EACH FACE of the truss member indicated.



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/def	L/d	Weight: 30 lb
TCLL 20.0	Plates Increase 1.15	TC 0.85	Vert(LL) 0.29	2-4	>480	360		
TCDL 10.0	Lumber Increase 1.15	BC 0.42	Vert(TL) -0.56	2-4	>251	240		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.01	4	n/a	n/a		
BCDL 5.0	Code IBC2006/TPI2002	(Matrix)						

LUMBER  
TOP CHORD 2 X 4 SPF No.2  
BOT CHORD 2 X 4 SPF No.2

BRACING  
TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 8-9-10 oc bracing.

REACTIONS (lb/size) 2=621/0-3-8, 4=621/0-3-8  
Max Horz 2=79(LC 8)  
Max Uplift 2=-366(LC 8), 4=-366(LC 9)

FORCES (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/9, 2-6=-766/608, 3-6=-578/626, 3-7=-578/626, 4-7=-766/608, 4-5=0/9  
BOT CHORD 2-4=400/624

- NOTES
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=3.0psf; h=12ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
  - TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=40.0 psf (ground snow); Pf=33.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.2
  - Unbalanced snow loads have been considered for this design.
  - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 33.6 psf on overhangs non-concurrent with other live loads.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 366 lb uplift at joint 2 and 366 lb uplift at joint 4.
  - This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



**Warning! - Verify design parameters and read notes before use.**

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer - not the truss designer or truss engineer, per ANSI/TPI 1-2002 Section 2.

A copy of this design shall be furnished to the erection contractor. This design is for an individual building component (a truss). It is based on specifications provided by the component manufacturer and performed in accordance with latest edition of TPI 1, IBC/IRC, and NDS design standards. No responsibility is assumed for the accuracy of information provided by the truss designer. The building designer shall review loading and truss configuration to ensure that this design meets or exceeds minimum loading required by applicable local building codes. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI / TPI 1(National Design Standard for Metal Connected Wood Truss Construction) and BCSI 1-03 (Guide to Good Practice for Handling, Installation & Bracing of Metal Plate Connected Wood Trusses) from the Truss Plate Institute.

10-0233