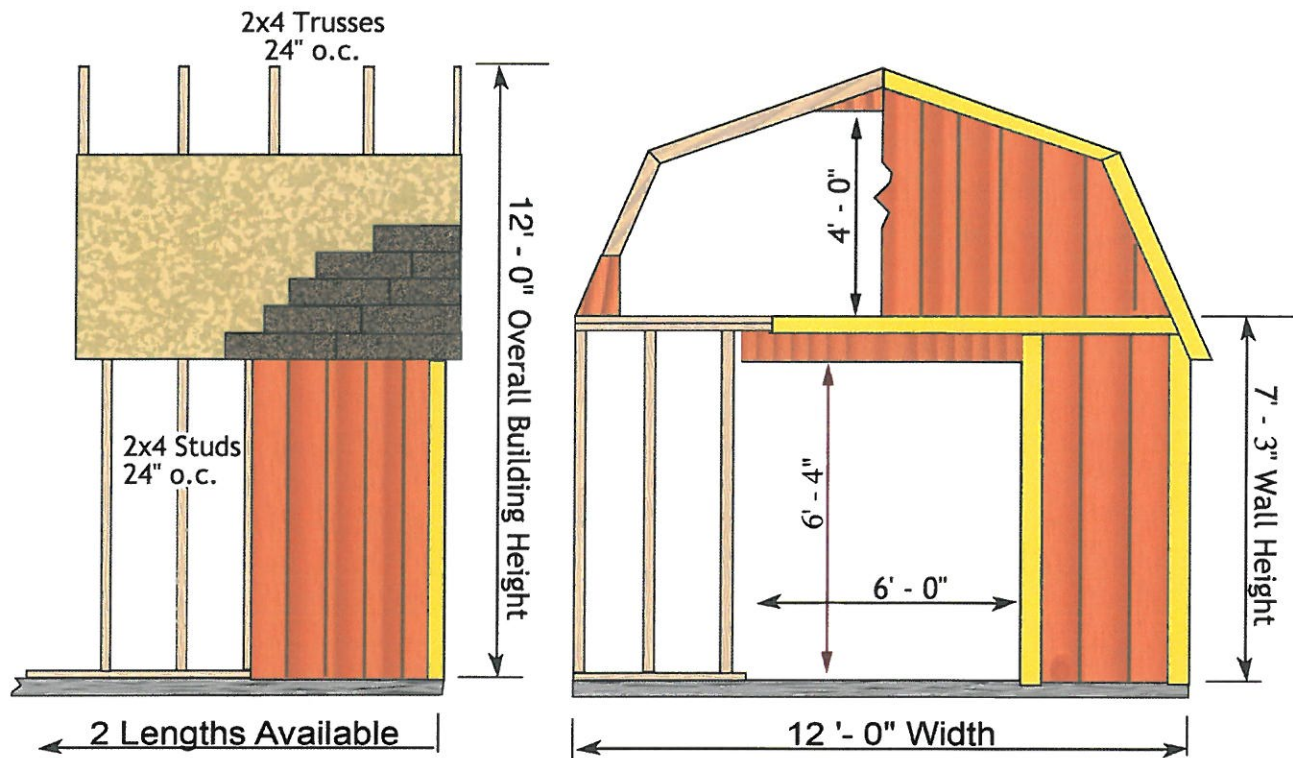




Before you order our kit or begin construction, obtain a building permit. The information below and the attached truss drawing should provide the information you will need.

If additional documents are required contact Richard@barnkits.com.

MILLCREEK ELEVATION



GENERAL SPECIFICATIONS

Foundation: By owner

Wall Framing: Constructed from 2x4 pre-cut wall studs spaced 24" o.c. Bottom plate, top and tie plate included.

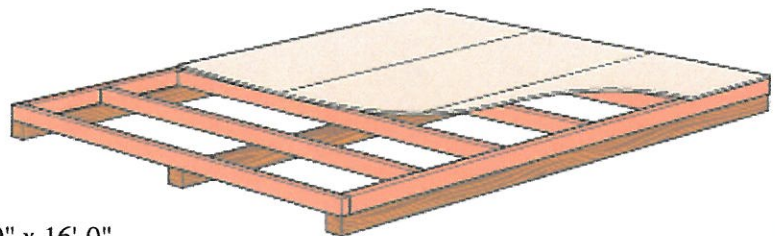
Siding: Louisiana-Pacific 'Smart Panel' primed 8" o.c. groove with 50 year warranty, 5 year labor replacement

Roof System: 2x4 trusses spaced 24" on center, (45 psf ground snow load, 120 mph wind load). 7/16" OSB roof sheathing. *Shingles by owner.*

Exterior Trim: White pine trim for corners, door and gable trim.

Hardware: Nails for all framing, metal hangers for trusses.

Optional Floor: 2x4 treated floor joist spaced 16" on center covered with 5/8" structurwood, installed over 4x4 treated runners. Note: 12' wide floors have four (4) treated runners. Nails are included. Material is not pre-cut.



12'x16' Foundation Size	12'-0" x 16'-0"
12'x20' Foundation Size	12'-0" x 20'-0"

Job	Truss	Truss Type	Qty	Ply	EZup Sheds - Sentry Building
10-0084_12FTGAMB	12GAMB-24ocs	ROOF TRUSS	1	1	

Koehlinger Engineering, Bolivar, OH 44612, (KDH)

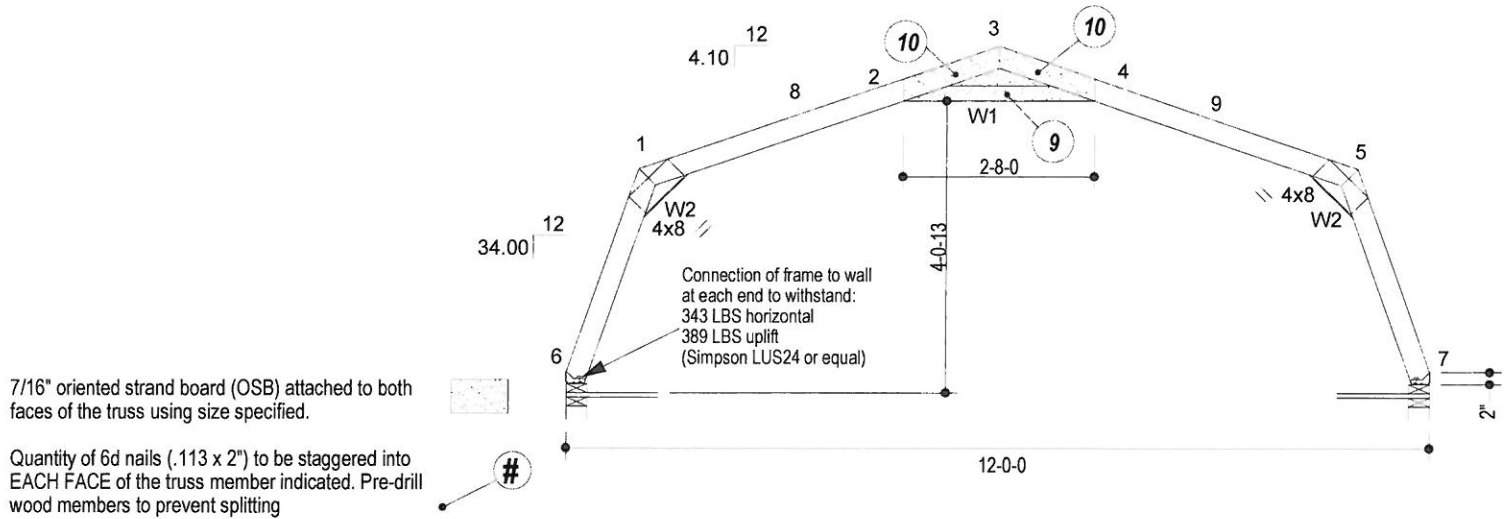


Plate Offsets (X,Y): [1:0-0-0,0-0-1], [3:0-2-0,Edge], [5:0-0-0,0-0-1]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.96	Vert(LL)	-0.24	1-2	>579	360	MT20	197/144
TCDL 12.0	Lumber Increase	1.15	BC 0.00	Vert(TL)	-0.34	4	>409	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.34	Horz(TL)	0.00		n/a	n/a		
BCDL 2.0	Code IRC2006/TPI2002		(Matrix)							Weight: 23 lb

LUMBER
TOP CHORD 2 X 4 SPF No.2
WEBS 2 X 3 SPF Stud

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=583/0-3-8, 7=583/0-3-8
Max Horz 6=343(LC 1), 7=-343(LC 1)
Max Uplift 6=-389(LC 7), 7=-389(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-6=-655/533, 1-8=-482/514, 2-8=-381/527, 2-3=-905/559, 3-4=-905/559, 4-9=-381/527, 5-9=-482/514,
5-7=-655/533
WEBS 2-4=-152/566

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 120mph; TCCL=7.2psf; BCCL=0.0psf; h=15ft; Cat. II; Exp C; partially; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=45.0 psf (ground snow); Pf=37.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.2
- 4) Unbalanced snow loads have been considered for this design.
- 5) The bottom chord dead load shown is sufficient only to cover the truss weight itself and does not allow for any additional load to be added to the bottom chord.
- 6) Plates checked for a plus or minus 0 degree rotation about its center.
- 7) Bearing at joint(s) 6, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 389 lb uplift at joint 6 and 389 lb uplift at joint 7.
- 9) Non Standard bearing condition. Review required.
- 10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

Warning! - Verify design parameters and read notes before use.

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer - not the truss designer or truss engineer, per ANSI/TPI 1-2002 Section 2.

A copy of this design shall be furnished to the erection contractor. This design is for an individual building component (a truss). It is based on specifications provided by the component manufacturer and performed in accordance with latest edition of TPI 1, IBC/IRC, and NDS design standards. No responsibility is assumed for the accuracy of information provided by the truss designer. The building designer shall review loading and truss configuration to ensure that this design meets or exceeds minimum loading required by applicable local building codes. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI / TPI 1(National Design Standard for Metal Connected Wood Truss Construction) and BCSI 1-03 (Guide to Good Practice for Handling, Installation & Bracing of Metal Plate Connected Wood Trusses) from the Truss Plate Institute.

10-0084

